

### Final Exam

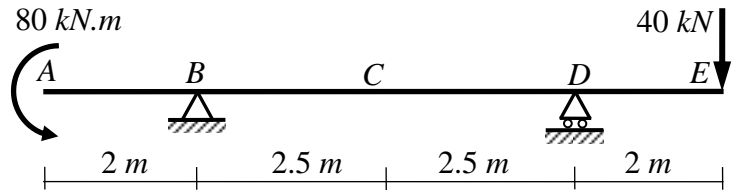
Total Marks: 70

No. of Questions: 40 (Attempt all questions)

#### Choose the nearest answer.

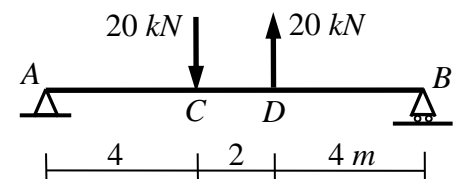
For the shown beam, it is required to determine the deflections at  $A$ ,  $C$  and  $E$  and the slope at  $A$  by using the **double integration method**.

$$EI = 5 \times 10^4 \text{ kN.m}^2$$

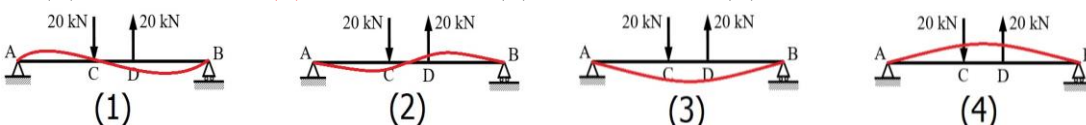


- The vertical reaction at the hinged support  $B$  is:  
(A) 40 kN  $\uparrow$  (B) 40 kN  $\downarrow$  (C) 80 kN  $\uparrow$  (D) zero
- The vertical reaction at the hinged support  $D$  is:  
(A) 40 kN  $\uparrow$  (B) 40 kN  $\downarrow$  (C) 80 kN  $\uparrow$  (D) 10 kN  $\downarrow$
- The bending moment equation ( $M$ ) in the last part  $DE$  is:  
(A)  $-80(x) + 40(x-7)$  (B)  $-80(x)^0 + 40(x-7)$  (C)  $-80(x)^2 + 40(x)$  (D)  $-80(x)^0 - 40(x)^2$
- $EIy' = \dots\dots$   
(A)  $-80(x) + 40(x-7)$  (B)  $-80x + 20(x-7)^2 + C_1$  (C)  $-80(x)^2 + 40(x)$  (D)  $-80(x)^0 - 40(x)^2$
- $EIy = \dots\dots$   
(A)  $-40(x)^2 + 20(x-7)^2 + C_1x$  (B)  $-80x + 20(x-7)^2 + C_1x + C_2$  (C)  $-40x^2 + 20(x-7)^3/3 + C_1x + C_2$  (D)  $-40 + C_1x + C_2$
- Boundary Conditions are:  
(A) At  $x=0, y=0$  & at  $x=9, y=0$  (B) At  $x=2, y=0$  & at  $x=9, y'=0$  (C) At  $x=2, y=0$  & at  $x=7, y=0$
- $C_1$  and  $C_2$  are:  
(A)  $C_1=360$  and  $C_2=-560$  (B)  $C_1=360$  and  $C_2=0$  (C)  $C_1=30$  and  $C_2=-50$  (D)  $C_1=0$  and  $C_2=-560$
- The deflection at  $A, y_A$  is:  
(A) 11.2 mm  $\uparrow$  (B) 5 mm  $\uparrow$  (C) zero (D) 11.2 mm  $\downarrow$
- The deflection at  $C, y_C$  is:  
(A) 11.2 mm  $\uparrow$  (B) 10.1 mm  $\uparrow$  (C) zero (D) 5 mm  $\uparrow$
- The deflection at  $E, y_E$  is:  
(A) 11.2 mm  $\uparrow$  (B) 10.1 mm  $\uparrow$  (C) zero (D) 10.1 mm  $\downarrow$
- The slope at  $A, \theta_A = y'_A$  is:  
(A) 0.0072 rad (B) 0.72 rad (C) 0.12 rad (D) 0.41 rad

For the shown beam, it is required to determine the slopes at  $A$  &  $C$  and the deflection at  $D$  by using the **moment-area method**.  $EI = 2.5 \times 10^3 \text{ kN.m}^2$

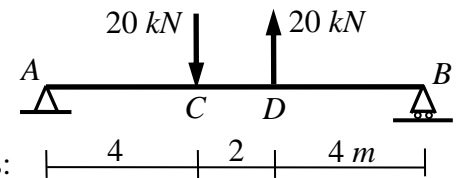


- The vertical reaction at the hinged support  $A$  is:  
(A) 20 kN  $\uparrow$  (B) 20 kN  $\downarrow$  (C) 4 kN  $\uparrow$  (D) 4 kN  $\downarrow$
- The bending moment at  $C$  is:  
(A) 32 kN.m (B) 16 kN.m (C) 40 kN.m (D) -16 kN.m
- The bending moment at  $D$  is:  
(A) 32 kN.m (B) 16 kN.m (C) 40 kN.m (D) -16 kN.m
- The deviation of  $B$  relative to the tangent of the elastic curve at  $A, t_{B/A}$  is:  
(A) 0.032 m (B) 0.016 m (C) 0.064 m (D) 0.046 m
- The slope of the tangent of the elastic curve at  $A, \theta_A$  is:  
(A) 0.0032 rad  $\cup$  (B) 0.0064 rad  $\cup$  (C) 0.0016 rad  $\cup$  (D) 0.0016 rad  $\cup$
- The slope of the tangent of the elastic curve at  $C, \theta_C$  is:  
(A) 0.0064 rad  $\cup$  (B) 0.0064 rad  $\cup$  (C) 0.0016 rad  $\cup$  (D) 0.0016 rad  $\cup$
- The deviation of  $D$  relative to the tangent of the elastic curve at  $A, t_{D/A}$  is:  
(A) 0.032 m (B) 0.024 m (C) 0.047 m (D) zero
- The deflection at  $D, \delta_D$  is:  
(A) 4.27 mm  $\uparrow$  (B) 8.53 mm  $\uparrow$  (C) zero (D) 4.27 mm  $\downarrow$
- The nearest elastic curve of the shown beam is:  
(A) 1 (B) 2 (C) 3 (D) 4



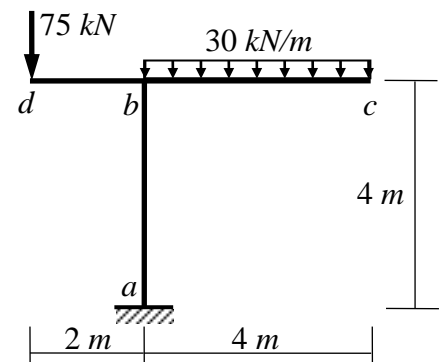
**Please turn over**

For the shown beam, it is required to determine the slopes at **B** & **D**, the deflection at **C** and the maximum deflection for the beam by using the **conjugate beam method**.  $EI = 2.5 \times 10^3 \text{ kN.m}^2$ .



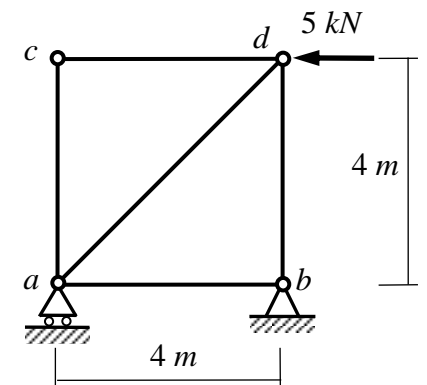
21. After loading  $M$  on the conjugate beam, the elastic reaction at support **A** is:  
 (A)  $32 \text{ kN.m}^2 \uparrow$  (B)  $64 \text{ kN.m}^2 \downarrow$  (C)  $32 \text{ kN.m}^2 \downarrow$  (D)  $16 \text{ kN.m}^2 \uparrow$
22. After loading  $M$  on the conjugate beam, the elastic reaction at support **B** is:  
 (A)  $32 \text{ kN.m}^2 \uparrow$  (B)  $16 \text{ kN.m}^2 \downarrow$  (C)  $32 \text{ kN.m}^2 \downarrow$  (D)  $15 \text{ kN.m}^2 \downarrow$
23. The slope of the tangent of the elastic curve at **B**,  $\theta_B$  is:  
 (A)  $0.0064 \text{ rad } \curvearrowright$  (B)  $0.0064 \text{ rad } \curvearrowleft$  (C)  $0.0016 \text{ rad } \curvearrowright$  (D)  $0.0016 \text{ rad } \curvearrowleft$
24. The slope of the tangent of the elastic curve at **D**,  $\theta_D$  is:  
 (A)  $0.0064 \text{ rad } \curvearrowright$  (B)  $0.0064 \text{ rad } \curvearrowleft$  (C)  $0.0016 \text{ rad } \curvearrowright$  (D)  $0.0016 \text{ rad } \curvearrowleft$
25. The deflection at **C**,  $\delta_C$  is:  
 (A)  $4.27 \text{ mm } \uparrow$  (B)  $8.53 \text{ mm } \uparrow$  (C) zero (D)  $8.53 \text{ mm } \downarrow$
26. The maximum downward deflection is at a distance from support **A** =:  
 (A)  $3.828 \text{ m}$  (B)  $5.00 \text{ m}$  (C)  $2.828 \text{ m}$  (D)  $0.046 \text{ m}$
27. The maximum downward deflection is:  
 (A)  $12.1 \text{ mm}$  (B)  $5.00 \text{ mm}$  (C)  $8.53 \text{ mm}$  (D)  $46.1 \text{ mm}$

For the shown frame, it is required to determine the vertical displacement at **d** ( $\delta_{dv}$ ) using the **virtual work method**.  $EI = 20 \times 10^3 \text{ kN.m}^2$ .



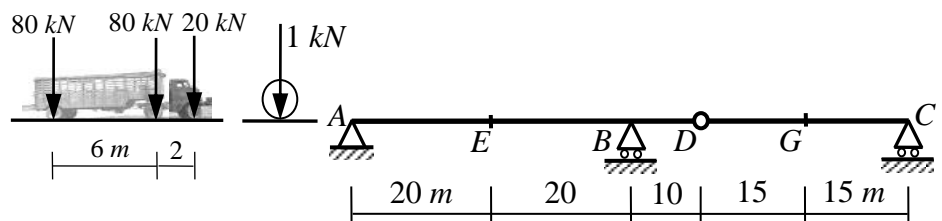
28. The bending moment due to the given load just at the left of **b** is:  
 (A)  $-75 \text{ kN.m}$  (B)  $-240 \text{ kN.m}$  (C)  $-60 \text{ kN.m}$  (D)  $-150 \text{ kN.m}$
29. The bending moment due to the given load ( $M_o$ ) just at the right of **b** is:  
 (A) zero (B)  $-240 \text{ kN.m}$  (C)  $-60 \text{ kN.m}$  (D)  $-150 \text{ kN.m}$
30. The bending moment due to unit load ( $M_I$ ) at **d** just at the right of **b** is:  
 (A) zero (B)  $-240 \text{ kN.m}$  (C)  $-60 \text{ kN.m}$  (D)  $-150 \text{ kN.m}$
31. The vertical displacement at **d**,  $\delta_{dv}$  is:  
 (A)  $5.2 \text{ mm } \uparrow$  (B)  $26 \text{ mm } \uparrow$  (C) zero (D)  $20 \text{ mm } \downarrow$

For the shown truss, it is required to determine the vertical displacement at **d** ( $\delta_{dv}$ ) using the **virtual work method**.  $EA = 1000 \text{ kN}$



32. The force in member **cd** due to the given load ( $N_o$ ) is:  
 (A) zero (B)  $-5 \text{ kN}$  (C)  $5 \text{ kN}$  (D)  $-7.1 \text{ kN}$
33. The force in member **bd** due to the given load ( $N_o$ ) is:  
 (A) zero (B)  $-5 \text{ kN}$  (C)  $5 \text{ kN}$  (D)  $-7.1 \text{ kN}$
34. The force in member **cd** due to the vertical unit load ( $N_I$ ) is:  
 (A) zero (B)  $-5 \text{ kN}$  (C)  $5 \text{ kN}$  (D)  $-7.1 \text{ kN}$
35. The vertical displacement at **d**,  $\delta_{dv}$  is:  
 (A)  $12 \text{ mm } \uparrow$  (B)  $20 \text{ mm } \uparrow$  (C) zero (D)  $20 \text{ mm } \downarrow$

For the shown beam, it is required to draw the influence line for the reaction  $C_y$  (*I.L.* for  $C_y$ ) and the bending moment at the section **G** (*I.L.* for  $M_G$ ). Also, determine the maximum moment at **G** caused by the shown moving truck.



36. The value of *I.L.* for  $C_y$  when the unit load at **C** and is:  
 (A) zero (B)  $0.5$  (C)  $1$  (D)  $1.5$
37. The value of *I.L.* for  $C_y$  when the unit load at **D** and is:  
 (A) zero (B)  $0.5$  (C)  $1$  (D)  $1.5$
38. The value of *I.L.* for  $M_G$  when the unit load at **C** and is:  
 (A) zero (B)  $0.5$  (C)  $1$  (D)  $1.5$
39. The value of *I.L.* for  $M_G$  when the unit load at **G** and is:  
 (A) zero (B)  $0.5$  (C)  $1$  (D)  $7.5$
40. The maximum moment at **G** caused by the shown moving truck is:  
 (A) zero (B)  $1090 \text{ kN.m}$  (C)  $190 \text{ kN.m}$  (D)  $-190 \text{ kN.m}$