

Answer of Final Exam

Total Marks: 60

No. of Questions: 6 (Attempt all questions)

Question (1): (10 Marks)

For the shown cross-section, determine the following:

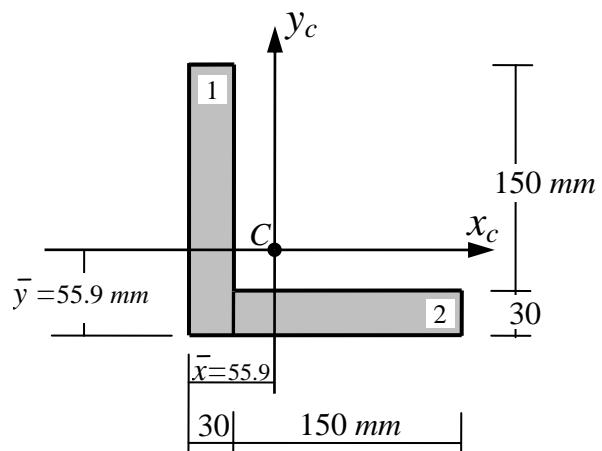
- (a) The location of the centroid.
- (b) The moments of inertia about the centroidal axes.
- (c) The direction of the principal axes.
- (d) The principal moments of inertia.

Solution:

(a) Location of the centroid:

$$\bar{x} = \frac{\sum A_i x_i}{A} = \frac{(180 \times 30)(15) + (150 \times 30)(105)}{180 \times 30 + 150 \times 30} = 55.9 \text{ mm}$$

$$\bar{y} = \frac{\sum A_i y_i}{A} = \frac{(180 \times 30)(90) + (150 \times 30)(15)}{180 \times 30 + 150 \times 30} = 55.9 \text{ mm}$$



(b) Moments of inertia about the centroidal axes:

$$I_{x_c} = \left[\frac{30 \times 180^3}{12} + (180 \times 30)(34.1)^2 \right] + \left[\frac{150 \times 30^3}{12} + (150 \times 30)(-40.9)^2 \right] \\ = 28724319 \text{ mm}^4$$

$$I_{y_c} = \left[\frac{180 \times 30^3}{12} + (180 \times 30)(-40.9)^2 \right] + \left[\frac{30 \times 150^3}{12} + (150 \times 30)(49.1)^2 \right] = 28724319 \text{ mm}^4$$

Note that the section is symmetrical about an axis makes an angle = 45° with the horizontal, therefore, you can calculate only \bar{x} and I_{x_c} , and then $\bar{y} = \bar{x}$ & $I_{y_c} = I_{x_c}$.

(c) Direction of the principal axes:

$$I_{x_c y_c} = [0 + (180 \times 30)(-40.9)(34.1)] + [0 + (150 \times 30)(49.1)(-40.9)] = -16568181 \text{ mm}^4$$

$$\tan 2\theta = \frac{-2I_{xy}}{(I_x - I_y)} = \frac{-2(-16568181)}{(28724319 - 28724319)} = \infty \quad \text{so, } 2\theta = 90^\circ \rightarrow \theta = 45^\circ$$

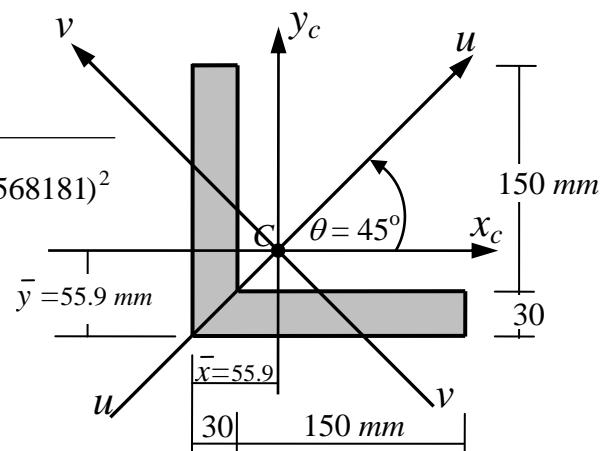
(d) Principal moments of inertia:

$$I_u = \frac{I_x + I_y}{2} \pm \sqrt{\left(\frac{I_x - I_y}{2}\right)^2 + I_{xy}^2}$$

$$I_u = \frac{28724319 + 28724319}{2} \pm \sqrt{\left(\frac{28724319 - 28724319}{2}\right)^2 + (-16568181)^2}$$

$$I_u = 45292500 \text{ mm}^4 = 45.29 \times 10^6 \text{ mm}^4$$

$$I_v = 12156138 \text{ mm}^4 = 12.16 \times 10^6 \text{ mm}^4$$



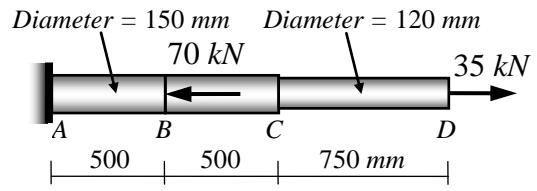
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Question (2): (10 Marks)

A stepped bar is subjected to axial loads as shown. Calculate the following:

- The normal stress in each part.
- The total elongation.
Where $E = 2.32 \text{ GPa}$



Solution:

- (a) σ in each part

For part AB:

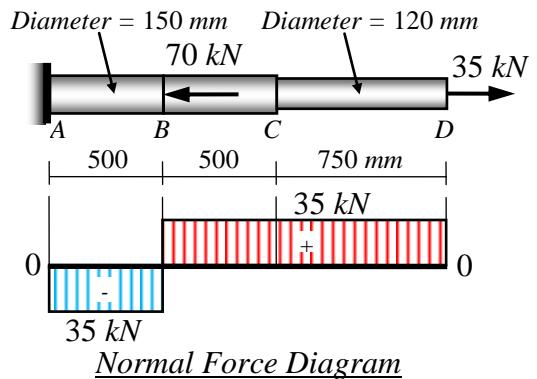
$$\sigma_{AB} = \frac{P_{AB}}{A_{AB}} = \frac{-35000}{\pi(75)^2} = -1.981 \text{ MPa}$$

For part BC:

$$\sigma_{BC} = \frac{P_{BC}}{A_{BC}} = \frac{35000}{\pi(75)^2} = 1.981 \text{ MPa}$$

For part CD:

$$\sigma_{CD} = \frac{P_{CD}}{A_{CD}} = \frac{35000}{\pi(60)^2} = 3.095 \text{ MPa}$$



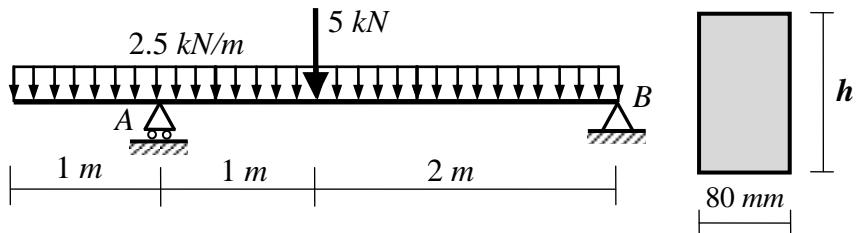
- (b) Total elongation δ

$$\delta = \sum \frac{PL}{EA} = \sum \frac{P L}{A E} = \sum \frac{\sigma L}{E} = \frac{1}{E} \sum \sigma L = \frac{1}{2320} [-1.981 \times 500 + 1.981 \times 500 + 3.095 \times 750] = 1.00 \text{ mm}$$

Question (3): (10 Marks)

Determine the minimum height h of the cross section of the beam loaded as shown. The maximum flexural stress, $f_{b\max} = 20 \text{ MPa}$.

Note: S.F.D and B.M.D are required.



Solution:

$$+\circlearrowleft \sum M_B = 0 :$$

$$A_y(3) - (2.5 \times 4)(2) - 5(2) = 0 \rightarrow A_y = 10 \text{ kN} \uparrow$$

$$+\uparrow \sum F_y = 0 :$$

$$A_y + B_y - (2.5 \times 4) - 5 = 0 \rightarrow B_y = 5 \text{ kN} \uparrow$$

$$I_x = \frac{80h^3}{12} = \frac{20}{3}h^3 \text{ mm}^3$$

$$M_{\max} = 5 \text{ kN.m} = 5(1000)(1000) \text{ N.mm}$$

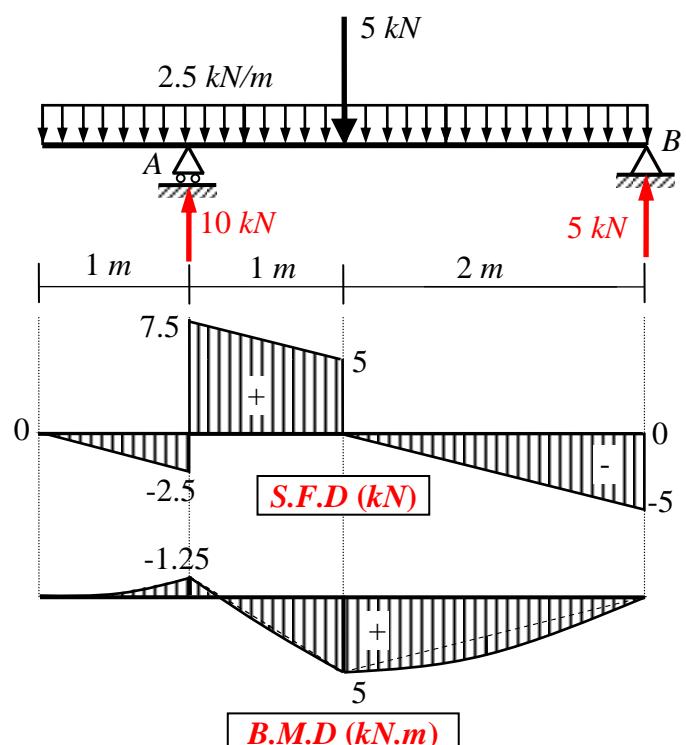
$$= 5 \times 10^6 \text{ N.mm}$$

$$y_{\max} = \frac{1}{2}h \text{ mm}$$

$$f_{b\max} = \frac{M_x}{I_x} y_{\max} \rightarrow 20 = \frac{5 \times 10^6}{\frac{20}{3}h^3} (\frac{1}{2}h)$$

$$\rightarrow h^2 = 18750 \text{ mm}^2 \rightarrow h = 136.93 \text{ mm}$$

$$\therefore h = 137 \text{ mm}$$

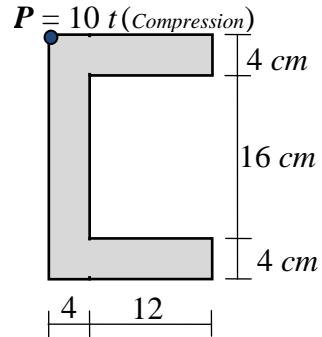


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Question (4): (10 Marks)

A cross-section is subjected to axial compressive load P as shown, calculate and draw the normal stress distribution over the cross-section.



Solution:

$$A = (4 \times 12) \times 2 + (4 \times 24) = 192 \text{ cm}^2$$

$$\bar{x} = \frac{\sum A_i x_i}{A} = \frac{2(4 \times 12)(8))}{192} = 4 \text{ cm}$$

$$I_{x_c} = 2 \left[\frac{12 \times 4^3}{12} + (12 \times 4)(10)^2 \right] + \left[\frac{4 \times 24^3}{12} \right] = 14336 \text{ } cm^4$$

$$I_{yc} = 2 \left[\frac{4 \times 12^3}{12} + (4 \times 12)(4)^2 \right] + \left[\frac{24 \times 4^3}{12} + (24 \times 4)(4)^2 \right] = 4352 \text{ } cm^4$$

$$N = -10 t$$

$$M_x = -10 \times 12 = -120 \text{ t.cm}$$

$$M_y = 10 \times 6 = 60 \text{ t.cm}$$

$$\sigma = \pm \frac{N}{A} \pm \frac{M_x}{I_x} y \pm \frac{M_y}{I_y} x = -\frac{10}{192} - \frac{120}{14336} y + \frac{60}{4352} x$$

N.A.

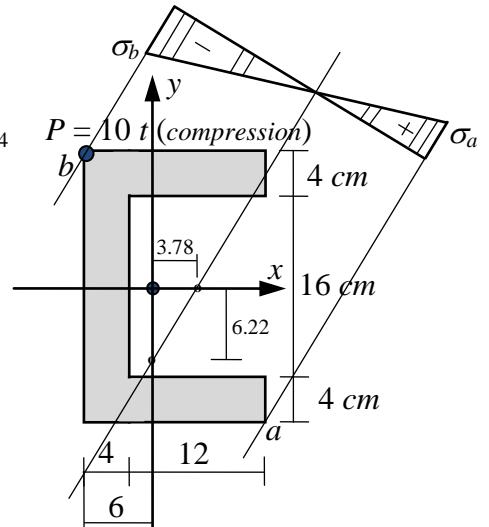
$$-\frac{10}{192} - \frac{120}{14336}y + \frac{60}{4352}x = 0$$

$$\text{At } x = 0; \quad y = -\frac{10 \times 14336}{192 \times 120} = -6.22 \text{ cm} \quad \rightarrow (0, -6.22)$$

$$\text{At } y = 0; \quad x = +\frac{10 \times 4352}{192 \times 60} = +3.78 \text{ cm} \rightarrow (-3.78, 0)$$

$$\sigma_a = -\frac{10}{192} - \frac{120}{14336}(-12) + \frac{60}{4352}(10) = 0.186 \quad = \boxed{+ 0.19 \text{ t/cm}^2}$$

$$\sigma_b = -\frac{10}{192} - \frac{120}{14336}(12) + \frac{60}{4352}(-6) = -0.235 \text{ } \text{ } \boxed{-0.24 \text{ } t/cm^2}$$

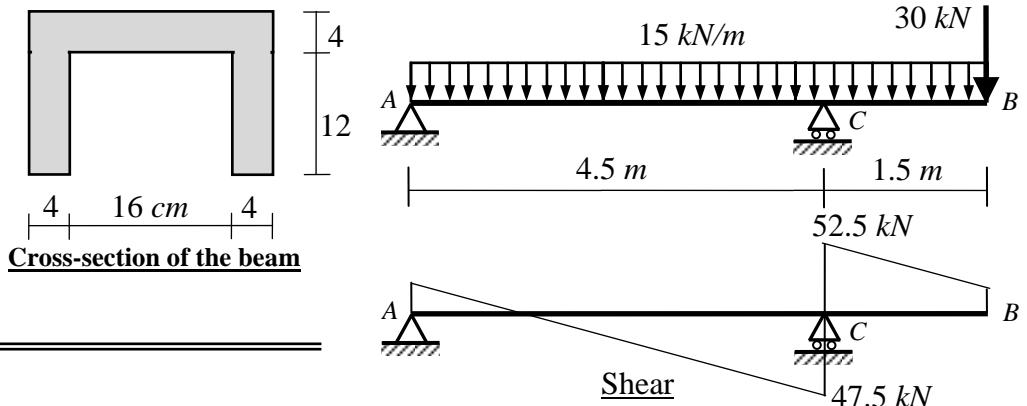
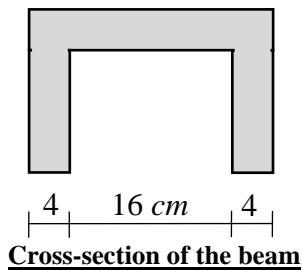


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Question (5): (10 Marks)

For the shown beam, calculate and draw the shear stress distribution over the cross-section at **C**.



Solution:

From the solution of Question (4), the location of the centroid of the section is known and

$$I_x \text{ here} = I_{y_c} = 4352 \text{ cm}^4$$

The reaction at the support **A** = 20 kN and at the support **C** = 100 kN

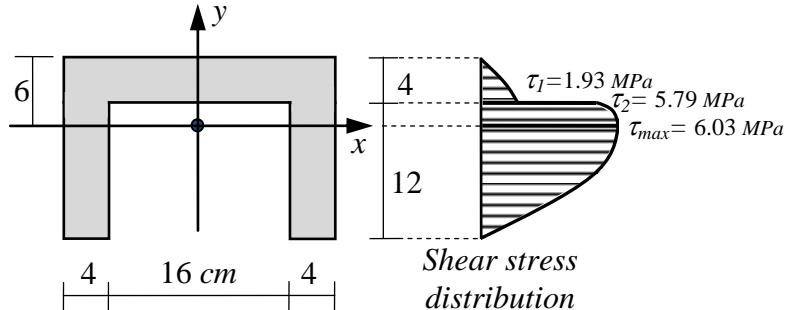
The shear force at section just right to **C**, $Q = 30 + 15 \times 1.5 = 52.5 \text{ kN}$

The shear force at section just left to **C**, $Q = 100 - 52.5 = 47.5 \text{ kN}$

$$\tau_1 = \frac{QS}{Ib} = \frac{(52.5)(24 \times 4)(4)}{4352 \times (24)} = 0.193 \text{ kN/cm}^2$$

$$\tau_2 = \frac{QS}{Ib} = \frac{(52.5)(24 \times 4)(4)}{4352 \times (4+4)} = 0.579 \text{ kN/cm}^2$$

$$\tau_{\max} = \frac{QS}{Ib} = \frac{(52.5)[(24 \times 4)(4) + 2(2 \times 4) \times 1]}{4352 \times (4+4)} = 0.603 \text{ kN/cm}^2$$



Question (6): (10 Marks)

A tube must resist a torque due to the wind load on the sign board. The wind pressure is 1.5 kN/m^2 . The tube has an outside diameter of 200 mm and a thickness of 5 mm. Determine:

- The maximum shear stress τ_{\max} in the tube at section **B** due to the torque.
- The relative angle of twist between **A** and **B**, where $G = 30 \text{ GPa}$

Solution:

- The maximum shear stress τ_{\max}

The torque due wind load, $T = \text{wind load} \times \text{arm}$

$$= [1.5(2 \times 1)] \times 1 = 3 \text{ kN.m} = 3 \times 10^6 \text{ N.mm}$$

The polar moment of inertia, $J = I_P$ for a hollow tube is:

$$J = \frac{\pi(r_o^4 - r_i^4)}{2} = \frac{\pi(100^4 - 95^4)}{2} = 29.14 \times 10^6 \text{ mm}^4$$

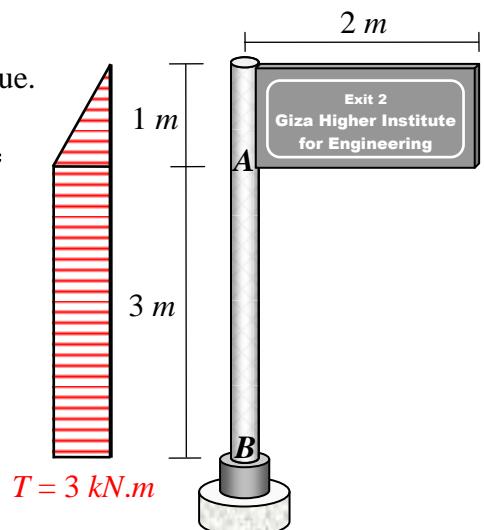
Therefore, the maximum shear stress

$$\tau_{\max} = \frac{Tr}{J} = \frac{3 \times 10^6 (100)}{29.14 \times 10^6} = 10.3 \text{ MPa}$$

The maximum shear stress is at the outside surface of the tube and has a magnitude of 10.3 MPa.

- The relative angle of twist between **A** and **B**:

$$\phi_{A/B} = \frac{TL}{JG} = \frac{3 \times 10^6 (3000)}{29.14 \times 10^6 (30 \times 10^3)} = 0.0103 \text{ rad} = 0.59^\circ$$



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