

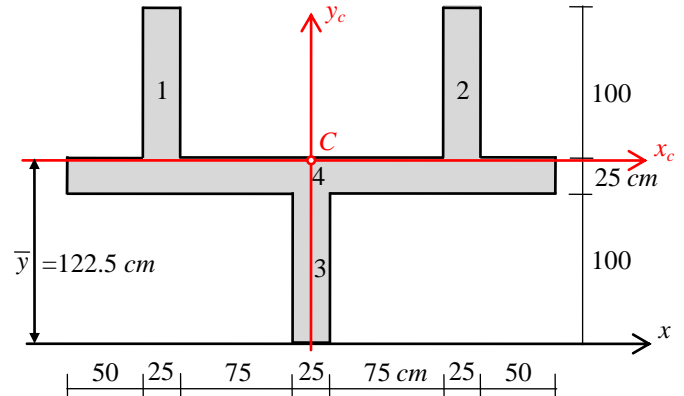
Answer of Second Semester Final Exam

Question (1): (12 Marks)

For the shown cross-section, determine the following:

- The location of the centroid.
- The moments of inertia about the centroidal axes (I_{x_c} & I_{y_c}).

Note: Divide the cross-section to 4 elements as shown on the figure.



Element	A_i	y_i	$A_i y_i$	I_{xi}	$dy_i = y_i - \bar{y}$	$A_i (dy_i)^2$	I_{yi}	dx_i	$A_i (dx_i)^2$
1	25×100	175	437500	25×100 ³ /12	52.5	6890625	100×25 ³ /12	-100	25000000
2	25×100	175	437500	25×100 ³ /12	52.5	6890625	100×25 ³ /12	100	25000000
3	25×100	50	125000	25×100 ³ /12	-72.5	13140625	100×25 ³ /12	0	0
4	25×325	112.5	914062.5	325×25 ³ /12	-10	812500	25×325 ³ /12	0	0
Σ	15625	--	1914062.5	6673177	--	27734375	71907552		50000000

(a) Location of the centroid:

- From symmetry y_c -axis is as shown

$$\bar{y} = \frac{\sum A_i y_i}{\sum A_i} = \frac{1914062.5}{15625} = \boxed{122.5 \text{ cm}}$$

(b) The moments of inertia about the centroidal axes

$$I_{x_c} = \sum (I_{xi} + A_i (dy_i)^2) = \sum I_{xi} + \sum A_i (dy_i)^2 = 6673177 + 27734375 = \boxed{34407552} = \boxed{34.4 \times 10^6 \text{ cm}^4}$$

$$I_{y_c} = \sum (I_{yi} + A_i (dx_i)^2) = \sum I_{yi} + \sum A_i (dx_i)^2 = 71907552 + 50000000 = \boxed{121907552} = \boxed{121.9 \times 10^6 \text{ cm}^4}$$

Question (2): (12 Marks)

Allowable **tensile** stress for concrete = 10 N/mm²

Allowable **compressive** stress for steel = 140 N/mm²

For Steel (Upper part):

$$\sigma_{Steel} = \frac{P_{Steel}}{A_{Steel}} = \frac{260 \times 10^3}{\pi(25)^2}$$

$$= 132.4 \text{ N/mm}^2 < 140 \text{ N/mm}^2 \rightarrow \text{Safe} \dots (1)$$

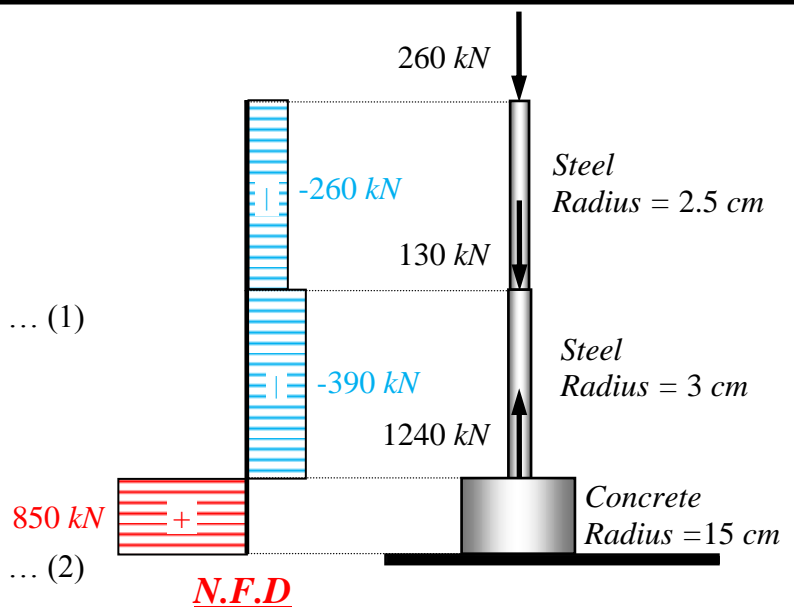
For Steel (Lower part):

$$\sigma_{Steel} = \frac{P_{Steel}}{A_{Steel}} = \frac{390 \times 10^3}{\pi(30)^2}$$

$$= 137.9 \text{ N/mm}^2 < 140 \text{ N/mm}^2 \rightarrow \text{Safe} \dots (2)$$

For Concrete:

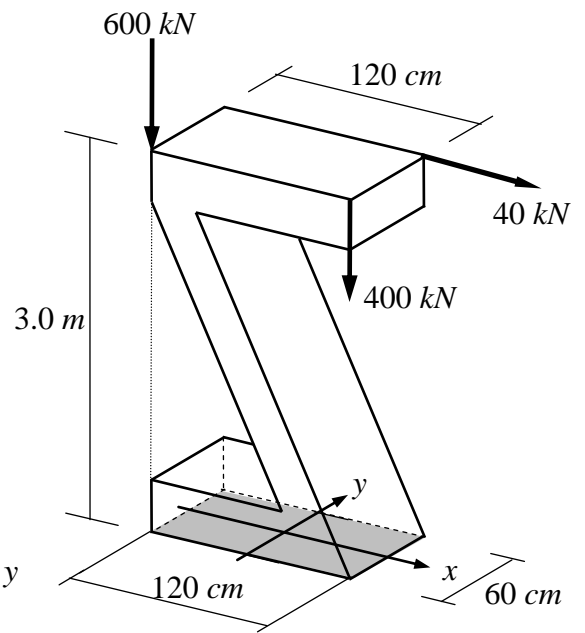
$$\sigma_{Con} = \frac{P_{Con}}{A_{Con}} = \frac{850 \times 10^3}{\pi(150)^2} = 12.0 \text{ N/mm}^2 > 10 \text{ N/mm}^2 \rightarrow \text{Unsafe} \dots (3)$$



\therefore From (1), (2) and (3), the column is **unsafe** to subject these axial loads.

Question (3): (12 Marks)

At the base section (120 cm × 60 cm) of the shown column, draw the normal stress distribution and calculate the maximum normal stresses. Neglect the column weight.



Solution:

$$N = -600 - 400 = -1000 \text{ kN}$$

$$M_x = +600 \times 30 + 400 \times 30 = 30000 \text{ kN.cm}$$

$$M_y = +600 \times 60 - 400 \times 60 - 40 \times 300 = 0$$

$$A = 120 \times 60 = 7200 \text{ cm}^2$$

$$I_x = 120 \times 60^3 / 12 = 2160000 \text{ cm}^4$$

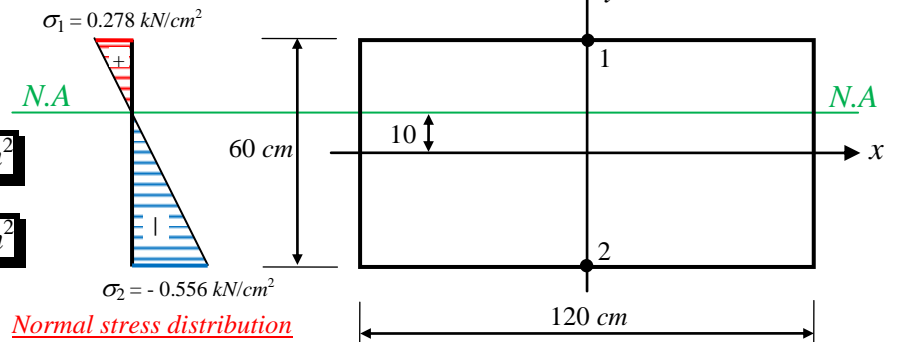
$$\sigma = \pm \frac{N}{A} \pm \frac{M_x}{I_x} y \pm \frac{M_y}{I_y} x = -\frac{1000}{7200} + \frac{30000}{2160000} y = -\frac{5}{36} + \frac{1}{72} y$$

N.A.

$$-\frac{5}{36} + \frac{1}{72} y = 0 \rightarrow y = 10 \text{ cm}$$

$$\sigma_1 = -\frac{5}{36} + \frac{1}{72} (30) = +0.278 \text{ kN/cm}^2$$

$$\sigma_2 = -\frac{5}{36} + \frac{1}{72} (-30) = -0.556 \text{ kN/cm}^2$$



Question (4): (12 Marks)

(a) The twisting moment diagram is as shown

(b)

Part AB

Twisting moment = $T = 50 \text{ kN.m} = 5000 \text{ kN.cm}$

$$J = \pi r^4 / 2 = \pi (5)^4 / 2 = 981.75 \text{ cm}^4$$

$$\text{So, } \tau = \frac{Tr}{J} = \frac{(5000)(5)}{981.75} = 25.46 \text{ kN/cm}^2$$

$$= 25.46 \text{ kN/cm}^2 = 254.6 \text{ MPa}$$

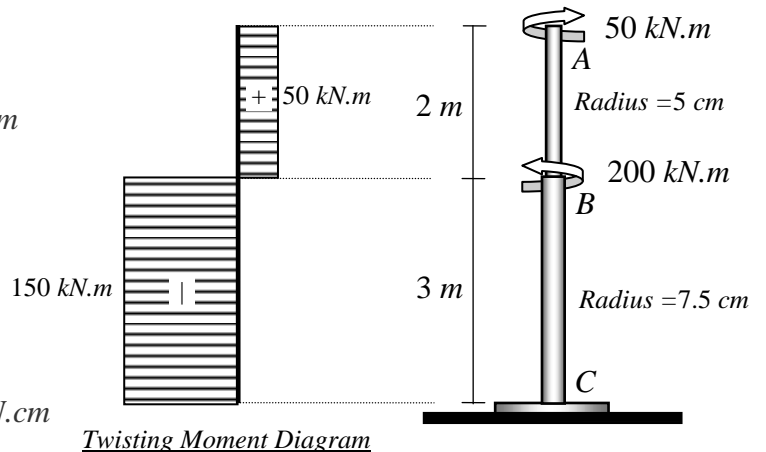
Part BC

Twisting moment = $T = 150 \text{ kN.m} = 15000 \text{ kN.cm}$

$$J = \pi r^4 / 2 = \pi (7.5)^4 / 2 = 4970 \text{ cm}^4$$

$$\text{So, } \tau = \frac{Tr}{J} = \frac{(15000)(7.5)}{4970}$$

$$= 22.64 \text{ kN/cm}^2 = 22.64 \text{ kN/cm}^2 = 226.4 \text{ MPa}$$



(c)

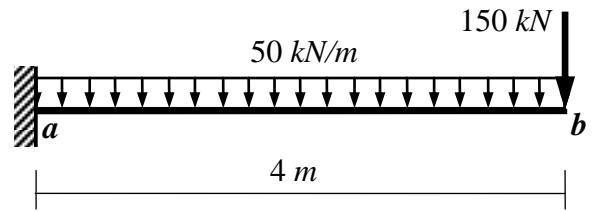
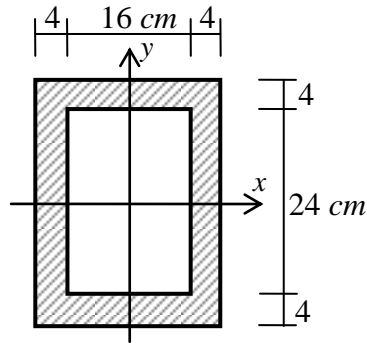
$$\phi_{A/C} = \phi_{A/B} + \phi_{B/C} = \frac{5000 \times 200}{981.75 \times 8000} + \frac{-15000 \times 300}{4970 \times 8000} = 0.1273 - 0.1132 = 0.0141 \text{ rad} = 0.82^\circ$$

With my best wishes
Dr. M. Abdel-Kader

Question (5): (12 Marks)

For the shown beam, calculate and draw:

- (a) The **normal** stress distribution over the cross-section at *a*.
- (b) The **shear** stress distribution over the cross-section at *a*.



Cross-section of the beam

Solution:

- (a) **Normal stress distribution over the cross-section at *b*.**

Bending moment at *a*, $M = 150 \times 4 + (50 \times 4) \times 2 = 1000 \text{ kN.m} = 100000 \text{ kN.cm}$

$$I = \frac{24 \times 32^3}{12} - \frac{16 \times 24^3}{12} = 47104 \text{ cm}^4$$

$$\sigma_t = \sigma_c = \frac{M}{I} y = \frac{100000}{47104} (16) = 33.97 \text{ kN/cm}^2 = 339.7 \text{ MPa}$$

Maximum tensile normal stress = Maximum compressive normal stress \cong **340 MPa**

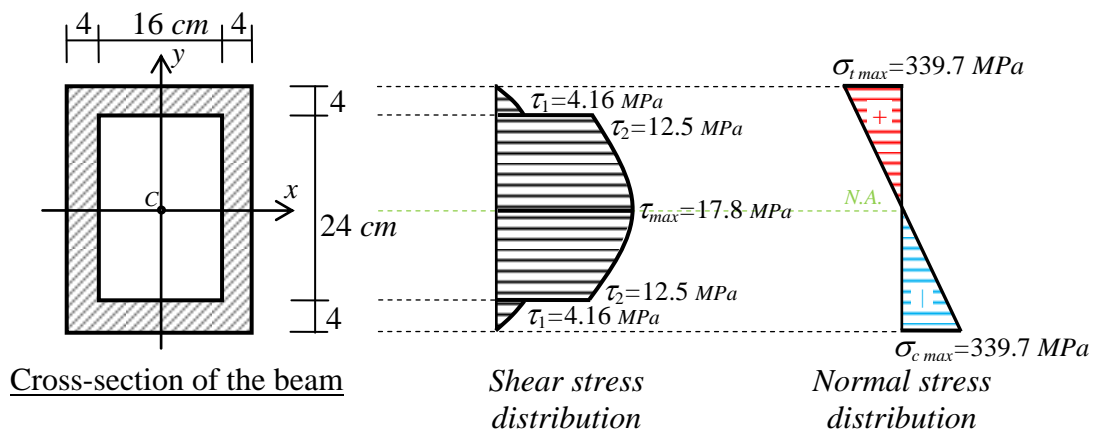
- (b) **Shear stress distribution over the cross-section at *a*.**

Shear force at *a*, $Q = 150 + 50 \times 4 = 350 \text{ kN} = 350 \times 10^3 \text{ N}$

$$\text{Shear stress, } \tau_1 = \frac{QS}{Ib} = \frac{(350 \times 10^3)(24 \times 4 \times 14)}{47104 \times 24} = 416 \text{ N/cm}^2 \quad \tau_1 = 4.16 \text{ MPa}$$

$$\text{Shear stress, } \tau_2 = \frac{QS}{Ib} = \frac{(350 \times 10^3)(24 \times 4 \times 14)}{47104 \times 8} = 1248 \text{ N/cm}^2 \quad \tau_2 = 12.5 \text{ MPa}$$

$$\text{Shear stress, } \tau_{\max} = \frac{QS}{Ib} = \frac{(350 \times 10^3)(24 \times 4 \times 14 + 2 \times 12 \times 4 \times 6)}{47104 \times 8} = 1783 \text{ kN/cm}^2 \quad \tau_{\max} = 17.8 \text{ MPa}$$



With my best wishes
Dr. M. Abdel-Kader